

Geotechnical

Environmental

Materials Testing

Construction Inspection

T. Y. Lin International/CCS 2010 Crow Canyon Place, Suite 350 San Ramon, CA 94583 August 10, 2006 Job No.: 205140.10

Attn.: Mr. Don Greb

Sub: Foundation Recommendations for Sound Wall 259

I-580 Vasco to Springtown Sound Wall Project

Livermore, Alameda County, California

Dear Mr. Greb:

As requested, we are providing the foundation recommendations for the proposed Sound Wall 259 as part of the I-580 Vasco to Springtown Sound Wall Project, in the City of Livermore, California.

Proposed Construction

The project consists of constructing a new sound wall approximately 300 meters long, and 5 meters high. The proposed sound wall will be on the northwest side of westbound I-580, approximately 0.9 km southwest of the intersection of I-580 and Vasco Road in the City of Livermore, California. Per discussion with the designer, it is planned to use standard Caltrans Sound Wall – Masonry Block on Type 736 Barrier. The structure will be supported on Cast-In-Drilled-Hole (CIDH) concrete piles.

Subsurface Conditions

Six borings were drilled up to 7.6 m deep below existing grade by Caltrans within the project vicinity in November 2001, and additional six new borings (259-1 thru 259-6) were drilled up to 9.1 m deep below grade by Parikh Consultants, Inc. in February 2006 for the proposed sound wall. Based on the boring data, the subsoils generally consist of stiff to hard cohesive material with occasional interbedded sand lenses. Soft to firm lean clays were encountered in Boring 259-1 (Sta. 219+03±) and 259-2 (Sta. 220+08±), located at the southwest end of the project.

Isolated loose to medium dense sand lenses up to 1.5 m thick were encountered in Boring 259-5 and 259-6 at approximately Elev. 154 m. These submerged sand lenses are subject to liquefaction during earthquakes. However, such loose sand lenses were not encountered in the nearby borings (B-7, B-9 and B-10, Caltrans 2001). In our opinion, post-liquefaction settlement on the order of 25 mm (1 inch) may be expected during strong earthquake, but probably would be random and localized.

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Based on the boring data, groundwater was encountered at approximate Elev. 155.1 to 157.5 m during field exploration. However, the groundwater level is anticipated to vary with the passage of time due to seasonal groundwater fluctuation, surface and subsurface flows, ground surface run-off, and other factors that may not be present at the time of investigation.

Foundations

The proposed sound wall is approximately 400 m long with wall height of 5.0 m. It is planned to use standard Caltrans Sound Wall – Masonry Block on Type 736 Barrier. The wall will be supported on barrier and CIDH concrete pile.

According to the boring data, the foundation subsoils generally consist of firm to hard fat/lean clay. Per discussion with the designer, the project will have various ground profiles of both level ground (Case 1) and sloping ground (Case 2) along the wall alignment. Based on the subsurface condition and ground profiles, an angle of shearing resistance (φ) of 30° is recommended for the standard Caltrans Sound Wall design. Please refer to Caltrans Standard Plans B15-6 thru B15-8 for foundation details.

The saturated loose sand lenses encountered in Borings 259-5 and 259-6 may be subject to liquefaction. As stated in the previous section, post-liquefaction settlement may be expected. The estimated settlement is on the order of 25 mm, and probably would be random and localized.

Caltrans standard specification for "Cast-in-Place Concrete Piling" should be used for the construction of CIDH concrete piles. The borings encountered sporadic sand lenses and the logs indicate groundwater at approximately 2.5 m below the existing grade. Sandy material and groundwater is expected during pile construction. Therefore, ravelling or caving might be expected which may require additional drilling and cleaning effort and may increase the concrete volume for the piles. The use of temporary steel casing should be expected. It is prudent to make the contractor aware of the presence of the groundwater so that he takes appropriate steps to comply with the standards and maintain the integrity of the piles.

Culverts

For Sound Wall No. 259, a drainage system is proposed along the wall alignment. It is recommended to wrap the pipe and the bedding with a layer of filter fabric conforming to Caltrans Standard Specification (Section 88-1.03). The culvert excavation and bedding material should follow Caltrans Standard Specification (Section 19-3.02). The bedding should be extended to at least 0.3 m (1 foot) above the pipe crown.



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Corrosion Investigation

Per Caltrans guidelines no specific geotechnical investigation is required for small diameter culverts (600 mm and under), and Standard Plans and Specifications can be used for design. During our investigation, three corrosion tests were performed along the proposed sound wall. The corrosion investigation for this project was performed in general accordance with the provisions of California Test Method 643. A summary of the corrosion test results is presented below.

SUMMARY OF CORROSION TEST RESULTS

Boring	Station (m)	Offset From "B9M" Line (m)	Corriosion Tests							
No.			Sample No.	Depth (m)	pН	Resistivity (ohm-cm)	Chloride (ppm)	Sulfate (ppm)		
259-3	221+13	Lt. 28.6	1	0.6	8.18	1130	19.1	14.5		
259-4	221+98	Lt. 24.1	1	0.6	7.52	1630	20.8	52.7		
259-5	223+00	Lt. 25.5	1	0.6	7.81	380	207.6	115.8		

Based on the results obtained, corrosion analyses were carried out using Caltrans CULVERT 4 program. The analysis results and general recommendations for culverts are attached in this report (Table 1).

Based on CULVERT 4 results, Standard reinforced concrete pipe design is suitable with Type IP (MS) modified cement or Type II Modified cement. This is a minimum requirement as per Caltrans Bridge Design Specifications (Section 8.22).

The thickness of corrugated steel and steel spiral rib pipes can vary from location to location. Based on the results, corrugated aluminum and corrugated aluminized steel pipe generally are not suitable within the project area. For steel pipes, 1.3 mm thick pipe can be used [50-yr., Galv. with polymeric coat. (90 invert)]. Thermoplastic pipe can be used as an alternative and should not have any corrosion concerns. However, the types of thermoplastic pipe that can be used will depend on the height of fill, available sizes and manufacturer's specifications.

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Please be advised that we are performing a professional service and that our conclusions are professional opinions only. All work done and all recommendations made are in accordance with generally accepted geotechnical engineering principles and practices. No warranty, expressed or implied, of merchantability or fitness, is made or intended in connection with our work.

Very truly yours,

PARIKH CONSULTANTS, INC.

Frank Y. Wang, P**(**É., 67751

Project Engineer

Gary Parikh, P.E., G.E., 66¢

roject Manager

Attachment:

1. Site Plan (1)

2. Log of Test Borings

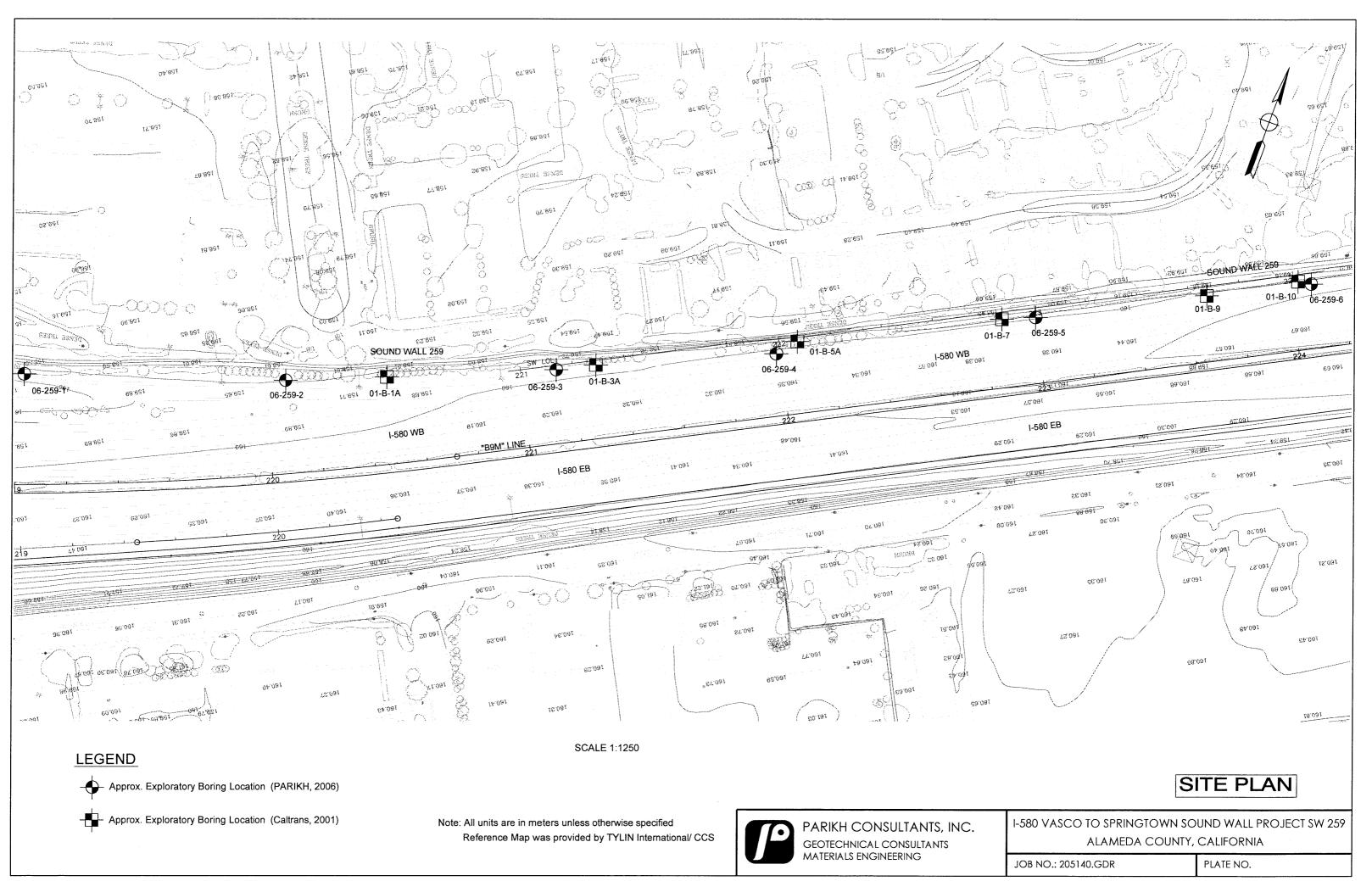
- 259-1 thru 259-6 (Parikh Consultants, 2006) (2)

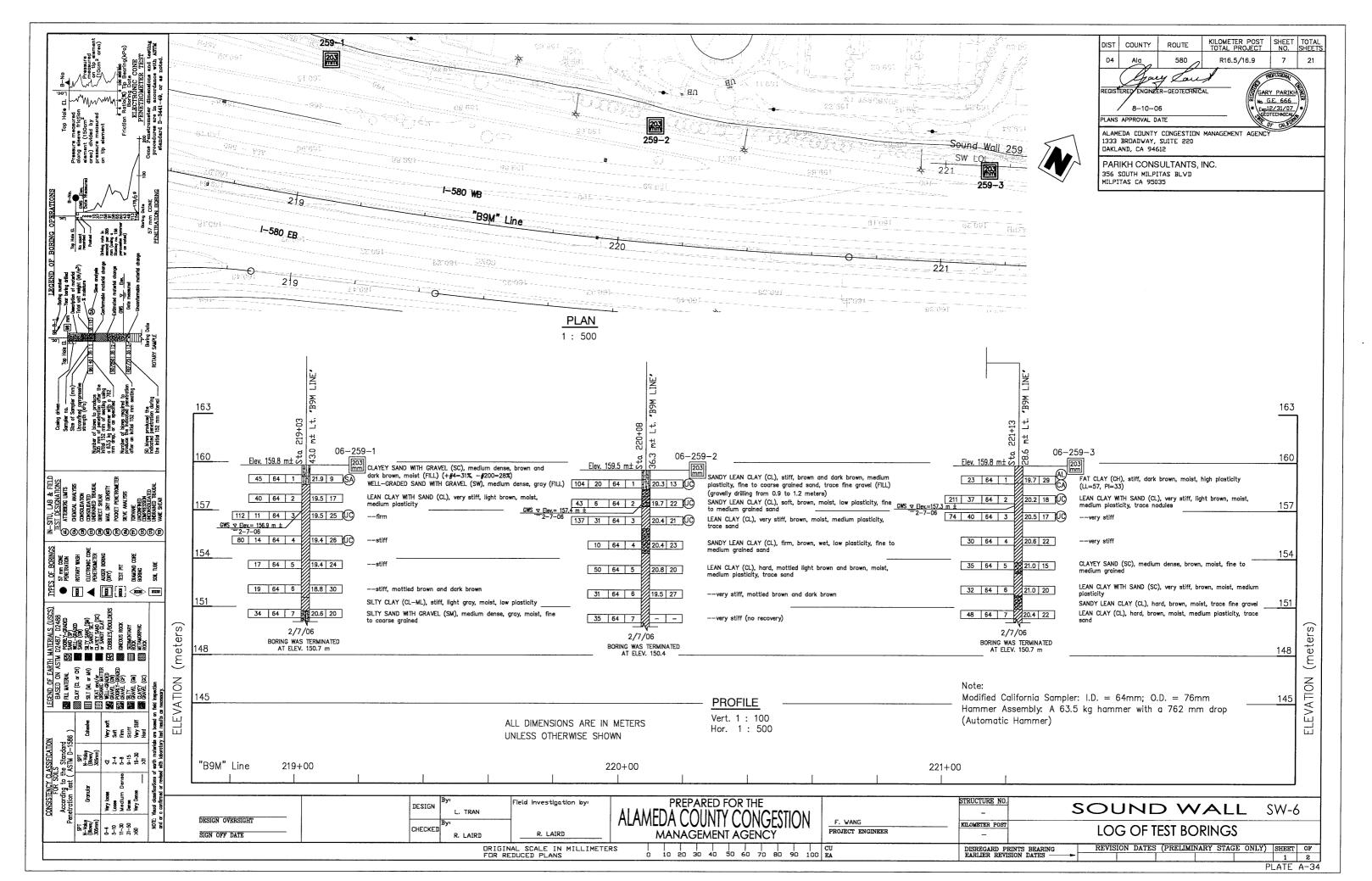
- B-1A, B-3A, B-5A, B-7, B-9 and B-10, (Caltrans, 2001) (2)

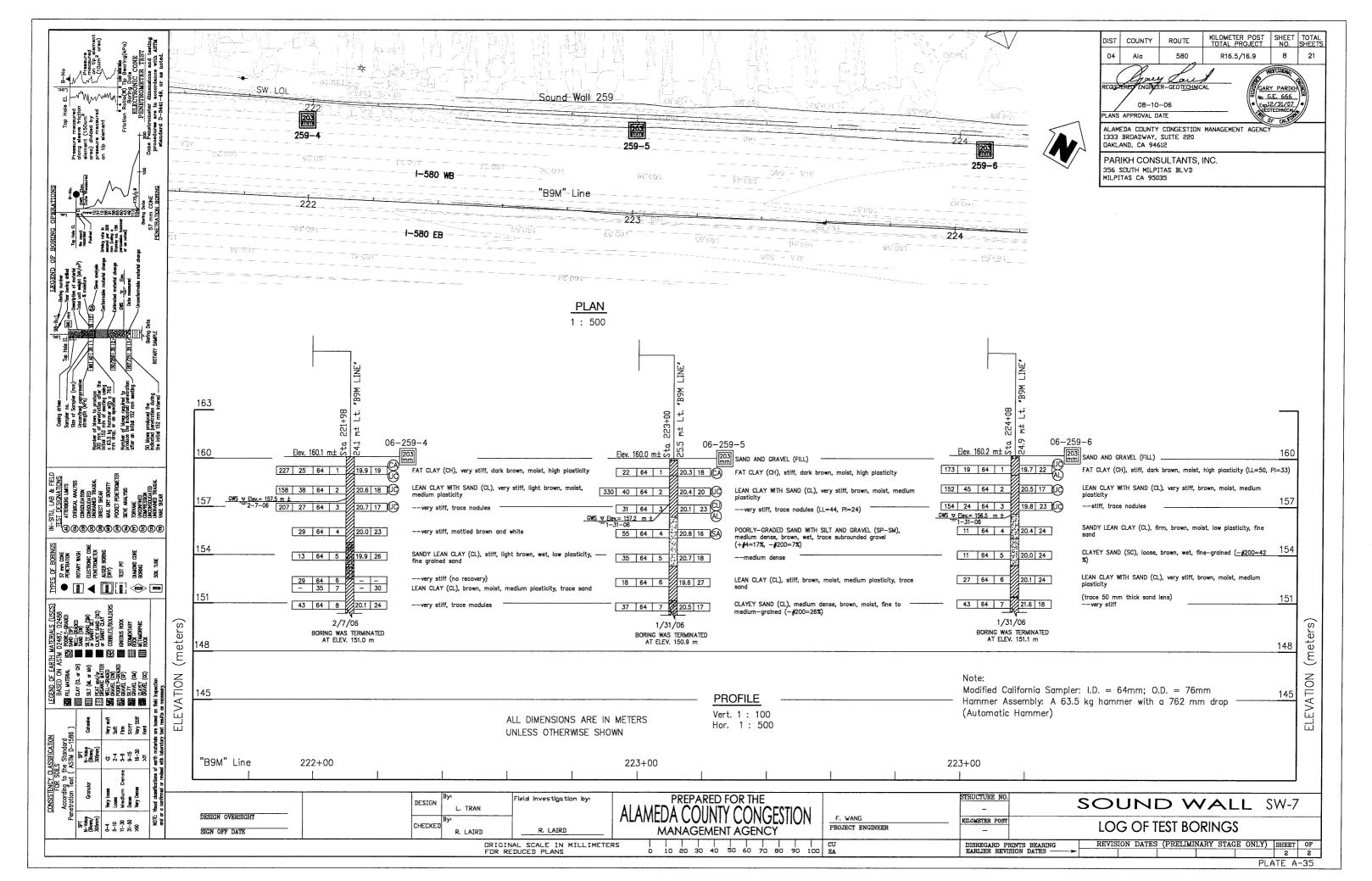
3. Table 1. Recommended Minimum Thickness and Protective Measures for Culverts (1)

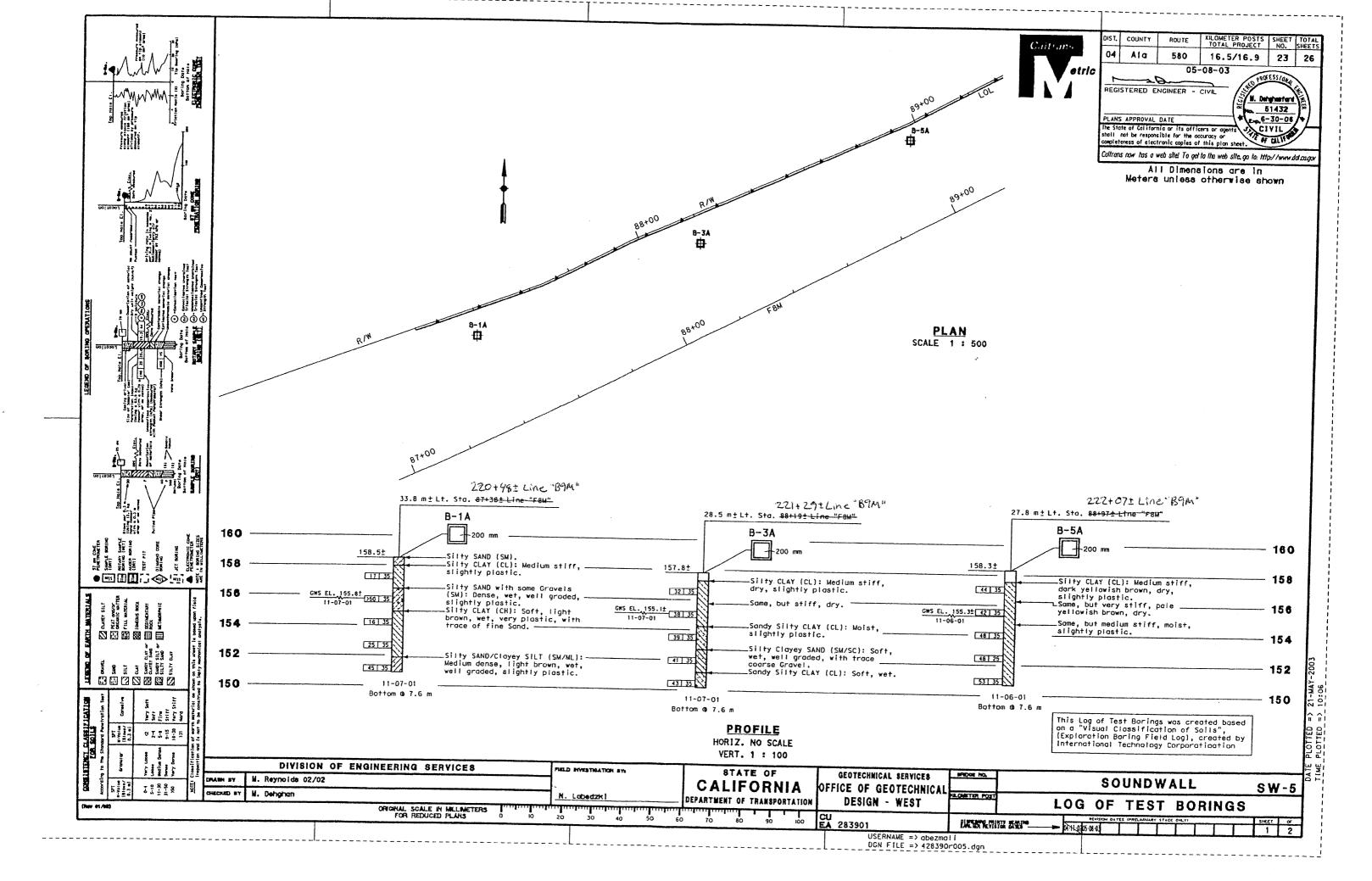
4. Culvert 4 Results (3)

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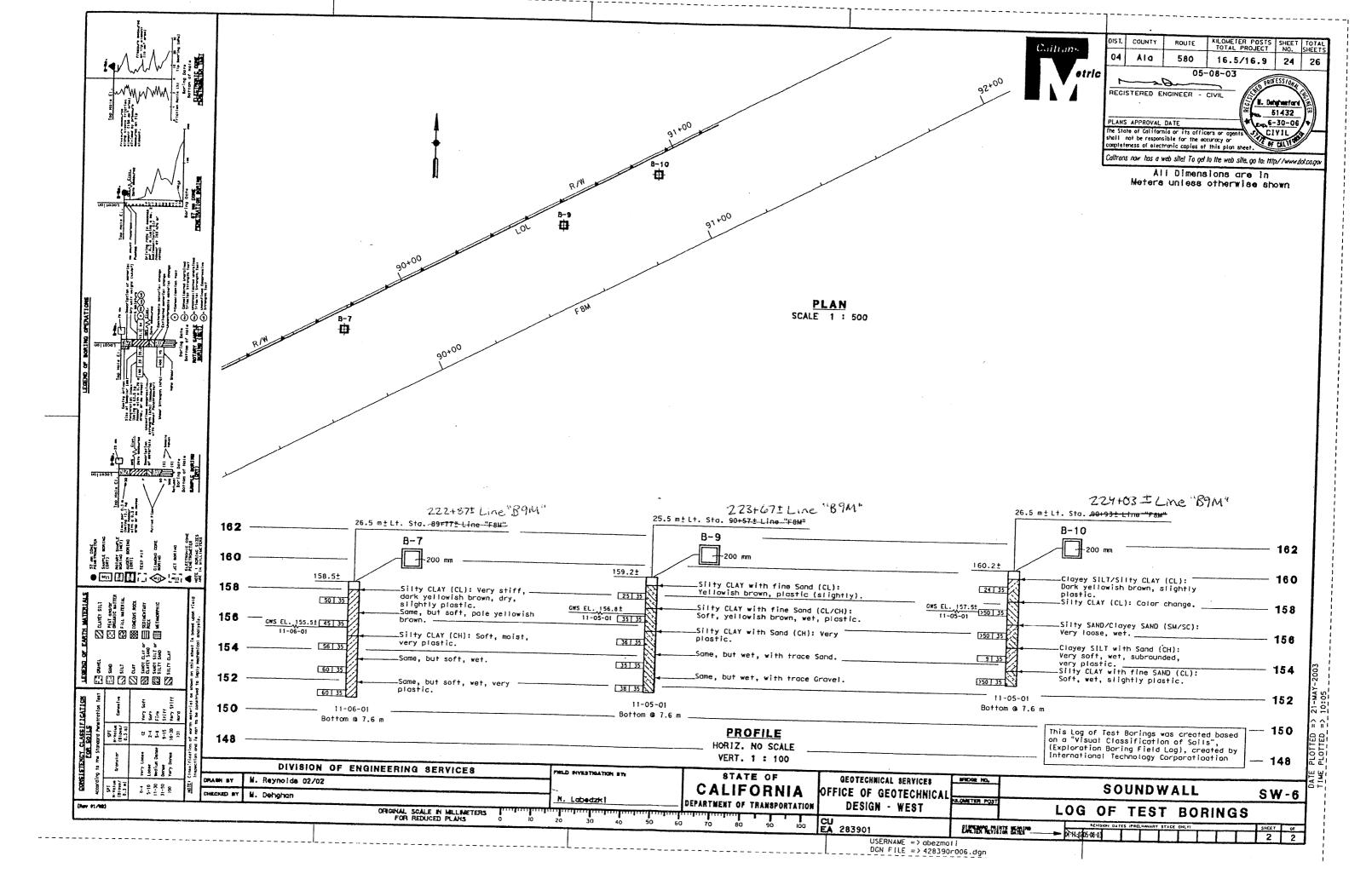


TABLE 1. RECOMMENDED MINIMUM THICKNESS AND PROTECTIVE MEASURES FOR CULVERTS

Location			Culvert 7	Гуре	Corrugated Steel / Steel Spiral Rib Pipe (Galv., mm)			Reinforced Concrete	Corrugated Aluminum (mm)	Corrugated Aluminized Steel (Type 2, mm)	Steel Pipe (Gal., mm)
			Est. Service I	Life (yr.)	25 50		50				50
Boring Number	Station &	& Offset*	Resistivity (ohms-cm)	PH	Galv	.(57g)	Bit. Coat. (Soil Sides)		50	50	Polymeric Coat. (90 Invert)
259-3	221+13	Lt. 28.6	1130	8.18	1.3	2.8	1.3	Note (1)	No	No	1.3
259-4	221+98	Lt. 24.1	1630	7.52	1.3	2.8	1.3	Note (1)	1.5	1.6	1.3
259-5	223+00	Lt. 25.5	380	7.81	2.0	4.3	2.0	Note (1)	No	No	1.3

Alternative Design: Thermoplastic pipe can be used as an alternative and should not have any corrosion concerns. However, the types of thermoplastic pipe can be used will depend on the height of fill, available sizes and manufacturer's specifications.

Note (1): Standard reinforced concrete pipe design is suitable with Type IP (MS) modified cement or Type II Modified cement, minimum required by Caltrans Std. Specs 90-1.01.

^{*} Ref. Line: "B9M" Line.

^{**}Maximum W/C ration of 0.45.

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MAINTENANCE-FREE SERVICE DESIGN ESTIMATES FOR DRAINAGE FACILITIES USING: CALIFORNIA CULVERT CRITERIA AND CULVERT4.EXE, (RELEASE DATE 04-16-98)

PROJECT LOCATION...I-580 HOV LANE

PROJECT ACCOUNT NO.205140.GDR

SAMPLE LOCATION....259-3

TEST SAMPLE NO....1

OPERATOR.....FYW

ESTIMATED SERVICE LIFE OF CSP CULVERTS, YEARS | SEE CALTRANS HIGHWAY DESIGN MANUAL CHAPTER 850

						
CSP THICK Gage & mm		GALV. 57 g	GALV.+ BIT COAT. (WATER SIDE)	GALV.+ BIT COAT & PAVED INV.	GALV.+ BIT COAT (SOIL SIDE)	GALV.+ POLYMER 90 DEG
18 16 14	1.3 1.6 2.0	 26 34 41	34 42 49	(ABRASION) 41 49 56	51 59 66	INVERT 76 84 91
12 10	2.8	57	65 81	72 88	82 98	107 123
8	4.3	89	97	104	114	139

FLOW VEL. <1.5 m/s WITH NON-ABRASIVE CONDITIONS, (DEFAULT VALUES) CAP, 18 GAGE (1.3 mm) CSP AND CASP MAY BE USED WITH THESE FLOW VELOCITIES

STANDARD REINFORCED CONCRETE PIPE DESIGN SHOULD BE SUITABLE FOR THIS USER DEFINED LEVEL OF CHLORIDES

CONCRETE AND RCP MITIGATION MEASURES FOR pH TYPE IP (MS) MODIFIED CEMENT OR TYPE II MODIFIED CEMENT MINIMUM REQUIRED BY CALTRANS STD. SPECS. 90-1.01

A CORRUGATED ALUMINUM PIPE, CAP, SHOULD NOT BE USED DUE TO CORROSIVE CONDITIONS

A CORRUGATED ALUMINIZED STEEL PIPE, CASP, SHOULD NOT BE USED DUE TO CORROSIVE CONDITIONS

PLASTIC PIPE IS APPROVED FOR 50 YEARS SERVICE LIFE FOR CORROSIVE CONDITIONS. ABRASION MUST BE EVALUATED. ALSO, CONSIDER CONCRETE HEADWALLS AND CONCRETE OR METAL END TREATMENT WHERE HIGH FIRE POTENTIAL EXISTS.

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MAINTENANCE-FREE SERVICE DESIGN ESTIMATES FOR DRAINAGE FACILITIES USING: CALIFORNIA CULVERT CRITERIA AND CULVERT4.EXE, (RELEASE DATE 04-16-98)

PROJECT LOCATION...I-580 HOV LANE

PROJECT ACCOUNT NO.205140.GDR

SAMPLE LOCATION....259-4

TEST SAMPLE NO....1

OPERATOR.....FYW

ESTIMATED SERVICE LIFE OF CSP CULVERTS, YEARS | SEE CALTRANS HIGHWAY DESIGN MANUAL CHAPTER 850

TH	CSP HICK e & mm	GALV. 57 g	GALV.+ BIT COAT. (WATER SIDE)	GALV.+ BIT COAT & PAVED INV. (ABRASION)	GALV.+ BIT COAT (SOIL SIDE)	GALV.+ POLYMER 90 DEG INVERT
18 16 14 12	1.3 1.6 2.0 2.8	30 39 48 67	38 47 56 75	45 54 63 82	55 64 73 92	80 89 98 117
$\overline{10}$	3.5	85	93	100	110	135
8	4.3	103	111	118	128	153

FLOW VEL. <1.5 m/s WITH NON-ABRASIVE CONDITIONS, (DEFAULT VALUES) CAP, 18 GAGE (1.3 mm) CSP AND CASP MAY BE USED WITH THESE FLOW VELOCITIES

STANDARD REINFORCED CONCRETE PIPE DESIGN SHOULD BE SUITABLE FOR THIS USER DEFINED LEVEL OF CHLORIDES

CONCRETE AND RCP MITIGATION MEASURES FOR pH TYPE IP (MS) MODIFIED CEMENT OR TYPE II MODIFIED CEMENT MINIMUM REQUIRED BY CALTRANS STD. SPECS. 90-1.01

A CORRUGATED ALUMINUM PIPE, CAP, MAY BE USED IF ABRASIVE CONDITIONS DO NOT EXIST SITE CONDITIONS MEET CORROSION REQUIREMENTS

A CORRUGATED ALUMINIZED STEEL PIPE, CASP, MAY BE USED SITE CONDITIONS MEET CORROSION REQUIREMENTS

PLASTIC PIPE IS APPROVED FOR 50 YEARS SERVICE LIFE FOR CORROSIVE CONDITIONS. ABRASION MUST BE EVALUATED. ALSO, CONSIDER CONCRETE HEADWALLS AND CONCRETE OR METAL END TREATMENT WHERE HIGH FIRE POTENTIAL EXISTS.

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MAINTENANCE-FREE SERVICE DESIGN ESTIMATES FOR DRAINAGE FACILITIES USING: CALIFORNIA CULVERT CRITERIA AND CULVERT4.EXE, (RELEASE DATE 04-16-98)

PROJECT LOCATION...I-580 HOV LANE

PROJECT ACCOUNT NO.205140.GDR

SAMPLE LOCATION....259-5

TEST SAMPLE NO....1

OPERATOR.....FYW

CSP SITE pH = 7.8, WATER pH = 0.0, SOIL pH = 7.8 MINIMUM RESISTIVITY, OHM-CM: CSP SITE = 380, WATER = 0, SOIL = 380

CHLORIDES, PPM... 207.6 , SULFATES, PPM... 115.8

ESTIMATED SERVICE LIFE OF CSP CULVERTS, YEARS | SEE CALTRANS HIGHWAY DESIGN MANUAL CHAPTER 850

CSP THICK Gage & mm		GALV. 57 g	GALV.+ BIT COAT. (WATER SIDE)	GALV.+ BIT COAT & PAVED INV.	GALV.+ BIT COAT (SOIL SIDE)	GALV.+ POLYMER 90 DEG
_		İ		(ABRASION)		INVERT
18	1.3	16	24	31	41	66
16	1.6	l 21	29	36	46	71
14	2.0	26	34	41	51	76
12	2.8	36	44	51	61	86
10	3.5	47	55	62	72	97
8	4.3	5 <i>7</i>	65	72	82	107

FLOW VEL. <1.5 m/s WITH NON-ABRASIVE CONDITIONS, (DEFAULT VALUES) CAP, 18 GAGE (1.3 mm) CSP AND CASP MAY BE USED WITH THESE FLOW VELOCITIES

STANDARD REINFORCED CONCRETE PIPE DESIGN SHOULD BE SUITABLE FOR THIS USER DEFINED LEVEL OF CHLORIDES

FOR SULFATE RESISTANT CONCRETE AND RCP TYPE IP (MS) MODIFIED CEMENT OR TYPE II MODIFIED CEMENT MINIMUM REQUIRED BY CALTRANS STD. SPECS. 90-1.01

A CORRUGATED ALUMINUM PIPE, CAP, SHOULD NOT BE USED DUE TO CORROSIVE CONDITIONS

A CORRUGATED ALUMINIZED STEEL PIPE, CASP, SHOULD NOT BE USED DUE TO CORROSIVE CONDITIONS

PLASTIC PIPE IS APPROVED FOR 50 YEARS SERVICE LIFE FOR CORROSIVE CONDITIONS. ABRASION MUST BE EVALUATED. ALSO, CONSIDER CONCRETE HEADWALLS AND CONCRETE OR METAL END TREATMENT WHERE HIGH FIRE POTENTIAL EXISTS.